

Figure 3.2 Effective stress shear strength envelopes: (a) for clay and (b) for sands, gravels, and rockfill.

As the upper half of the specimen moved to the left with respect to the lower half and the strength of the soil is mobilized, the particles within the rupture zone would be displaced from their original positions relative to adjacent particles. Any interparticle bonds would be broken, some individual particles may be broken, particles would rotate and be reoriented into new positions, and particles would slide across their contacts with neighboring particles. These movements of the particles would be resisted by the strength of any interparticle bonds, by frictional resistance to sliding, and by resistance to particle displacement and reorientation due to interference between particles as they are displaced. These types of resistance are the sources of shear strength in soils.

The two most important factors governing the strengths of soils are the magnitude of the interparticle contact forces and the density of the soil. Larger interparticle contact forces (larger values of effective stress) and higher densities result in higher strengths.

As τ increases, the shear displacement (Δx) between the top and the bottom of the shear box would increase, as shown in Figure 3.3. This shear displacement results from shear strains in the rupture zone. The shear displacements in direct

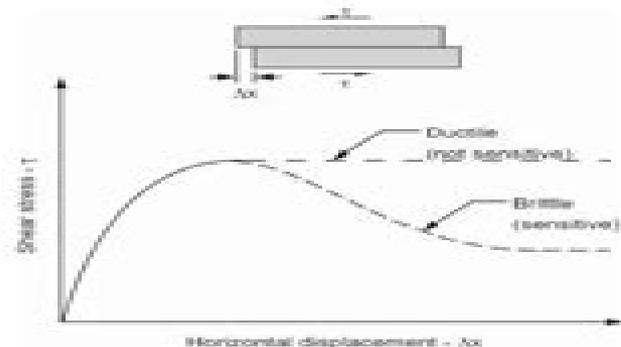


Figure 3.3 Shear stress–shear displacement curves for direct shear test.

shear tests can be measured easily, but shear strains cannot be determined because the thickness of the shear zone is not known. While the direct shear test can be used to measure the shear strengths of soils, it provides only qualitative information about stress–strain behavior. It is possible to determine whether soils are ductile (shear resistance remains high after failure) or brittle (shear resistance decreases after failure).

3.3.2 Drained Strength

Drained strength is the strength of the soil when it is loaded slowly enough so that there are no excess pore pressures induced by applied loads. In the field, drained conditions result when loads are applied to a mass of soil slowly, or where the loads persist for a long enough time so that the soil can drain and reach equilibrium with respect to water pressures. In the laboratory, drained conditions are achieved by loading test specimens slowly, so that excess pore pressures do not develop as the soil is loaded.

Imagine that the direct shear test specimen shown in Figure 3.1 reached a drained condition under the vertical load of 200 N and was then loaded to failure by increasing T slowly so that excess pore pressures did not develop. As shown by Eq. (3.11), the effective normal stress on the horizontal plane at equilibrium under the 200-N load would be equal to 20.1 kPa, and it would remain constant as the clay was sheared slowly.

The relationship between the shear strength of the specimen and the effective normal stress σ' is given by Eq. (3.13). If the clay is normally consolidated, c' will be equal to zero. The value of ϕ' would probably be between 25 and 35 degrees for normally consolidated sandy or silty clay. As an example, suppose that ϕ' is equal to 30 degrees. The drained strength

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Table of Contents Soil Strength And Slope Stability

1. Understanding the eBook Soil Strength And Slope Stability
 - The Rise of Digital Reading Soil Strength And Slope Stability
 - Advantages of eBooks Over Traditional Books
2. Identifying Soil Strength And Slope Stability
 - Exploring Different Genres
 - Considering Fiction vs. Non-Fiction
 - Determining Your Reading Goals
3. Choosing the Right eBook Platform
 - Popular eBook Platforms
 - Features to Look for in an Soil Strength And Slope Stability
 - User-Friendly Interface
4. Exploring eBook Recommendations from Soil Strength And Slope Stability
 - Personalized Recommendations
 - Soil Strength And Slope Stability User Reviews and Ratings
 - Soil Strength And Slope Stability and Bestseller Lists
5. Accessing Soil Strength And Slope Stability Free and Paid eBooks
 - Soil Strength And Slope Stability Public Domain eBooks
 - Soil Strength And Slope Stability eBook Subscription Services

- Soil Strength And Slope Stability Budget-Friendly Options
- 6. Navigating Soil Strength And Slope Stability eBook Formats
 - ePub, PDF, MOBI, and More
 - Soil Strength And Slope Stability Compatibility with Devices
 - Soil Strength And Slope Stability Enhanced eBook Features
- 7. Enhancing Your Reading Experience
 - Adjustable Fonts and Text Sizes of Soil Strength And Slope Stability
 - Highlighting and Note-Taking Soil Strength And Slope Stability
 - Interactive Elements Soil Strength And Slope Stability
- 8. Staying Engaged with Soil Strength And Slope Stability
 - Joining Online Reading Communities
 - Participating in Virtual Book Clubs
 - Following Authors and Publishers Soil Strength And Slope Stability
- 9. Balancing eBooks and Physical Books Soil Strength And Slope Stability
 - Benefits of a Digital Library
 - Creating a Diverse Reading Collection Soil Strength And Slope Stability
- 10. Overcoming Reading Challenges
 - Dealing with Digital Eye Strain
 - Minimizing Distractions
 - Managing Screen Time
- 11. Cultivating a Reading Routine Soil Strength And Slope Stability
 - Setting Reading Goals Soil Strength And Slope Stability
 - Carving Out Dedicated Reading Time
- 12. Sourcing Reliable Information of Soil Strength And Slope Stability
 - Fact-Checking eBook Content of Soil Strength And Slope Stability
 - Distinguishing Credible Sources
- 13. Promoting Lifelong Learning
 - Utilizing eBooks for Skill Development
 - Exploring Educational eBooks
- 14. Embracing eBook Trends

- Integration of Multimedia Elements
- Interactive and Gamified eBooks

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